

DE Karn History of Construction Bottom Ash Pond

Initial Compiled History Certification by Owner or Operator

Contents

Cert	ification Statement by Owner or Operator	3
1.0	Introduction	4
2.0	40 CFR 257.73 (c)(1)(i)	4
3.0	40 CFR 257.73 (c)(1)(ii)	4
4.0	40 CFR 257.73 (c)(1)(iii)	4
5.0	40 CFR 257.73 (c)(1)(iv)	5
6.0	40 CFR 257.73 (c)(1)(v)	5
7.0	40 CFR 257.73 (c)(1)(vi)	5
7.1	Physical and Engineering Properties	5
7.2	Site Preparation and Construction	6
8.0	40 CFR 257.73 (c)(1)(vii)	6
9.0	40 CFR 257.73 (c)(1)(viii)	7
10.0	40 CFR 257.73 (c)(1)(ix)	7
11.0	40 CFR 257.73 (c)(1)(x)	7
11.1	Spillway and Diversion Description	7
11.2	Capacities and Calculations	7
12.0	40 CFR 257.73 (c)(1)(xi)	8
12.1	Construction Specifications	8
12.2	Surveillance, Maintenance, and Repair	8
13.0	40 CFR 257.73 (c)(1)(xii)	3
14.0	Attachments	3
15.0	References	a

CERTIFICATION

Certification Statement by Owner or Operator

I certify under penalty of law that I have personally examined and am familiar with the information submitted in this demonstration and all attached documents, and that, based on my inquiry of those individuals immediately responsible for obtaining the information, I believe that the submitted information is true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment.

Consumers Energy Company

Signature

October 17, 2016

Date of Report Certification

Harold D. Register, Jr.

Name

Rev. 0_Signed Page 3 of 9

1.0 INTRODUCTION

The United States Environmental Protection Agency (EPA) promulgated the Resource Conservation and Recovery Act (RCRA) Coal Combustion Residuals (CCR) Rule ("CCR RCRA Rule") on April 17, 2015. The CCR RCRA Rule requires that owners or operators of existing CCR surface impoundments with a height of five feet or more and a storage volume of 20 acre-feet or more compile a history of construction, which shall contain, to the extent feasible, the information specified in 40 CFR 257.73 (c)(1)(i) through (xii). The history of construction, and any revisions of it, as required by 40 CFR 257.73(c) shall be placed in the operating record and shall be maintained until the CCR unit completes closure of the unit in accordance with 40 CFR 257.102 [40 CFR 257.105(f)(9)].

2.0 40 CFR 257.73 (C)(1)(I)

The name and address of the person(s) owning or operating the CCR unit; the name associated with the CCR unit; and the identification number of the CCR unit if one has been assigned by the state.

Consumers Energy Company Contact: Caleb Batts 2742 North Weadock Highway Essexville, MI 48732

Name of CCR Unit: DE Karn Bottom Ash Pond State Assigned Identification Number: None

3.0 40 CFR 257.73 (C)(1)(II)

The location of the CCR unit identified on the most recent U.S. Geological Survey (USGS) 7½ minute or 15 minute topographic quadrangle map, or a topographic map of equivalent scale if a USGS map is not available.

Figure 1 – Site Location Map presents the 7 ½ minute USGS quadrangle map Bay City NE, Michigan dated 2014. The location of the CCR unit is denoted on the map with the callout box – Site Location.

4.0 40 CFR 257.73 (C)(1)(III)

A statement of the purpose for which the CCR unit is being used.

According to the "<u>Potential Failure Mode Analysis (PFMA) Report</u>" prepared by AECOM (2009), DE Karn power generation facility consists of two coal-fired electric generating units that were put into service in 1959 (Unit 1) and 1961 (Unit 2), and two oil and gas co-fired units that were added in 1975 (Unit 3) and 1977 (Unit 4). The facility is bound by Saginaw Bay to the north and east, the Saginaw River to the west, and the former J.C. Weadock generation facility to the south.

An elevated trestle and pipe system hydraulically conveys sluiced bottom ash to the Bottom Ash Pond. Stored bottom ash is removed from the pond via mechanical equipment as required to maintain storage

Rev. 0_Signed Page 4 of 9

capacity. When an overflow condition exists, water is discharged from the Bottom Ash Pond via one 24-inch diameter steel pipe into a series of ditches that convey the flow to the permitted National Pollutant Discharge Elimination System (NPDES) outfall.

5.0 40 CFR 257.73 (C)(1)(IV)

The name and size in acres of the watershed within which the CCR unit is located.

The CCR RCRA Rule requires the name and size (in acres) of the watershed within which the CCR surface impoundment is located. According to the EPA MyWATERS Mapper website (USEPA 2016), the CCR surface impoundment is located within the Walther Drain-Frontal Lake Huron Subwatershed, which encompasses approximately 15,250 acres.

6.0 40 CFR 257.73 (C)(1)(V)

A description of the physical and engineering properties of the foundation and abutment materials on which the CCR unit is constructed.

As part of a subsurface investigation and sampling program conducted by Golder in 2015 and 2016, soil samples were collected from adjacent locations and from beneath the Bottom Ash Pond, respectively. Sampling locations are visually depicted on **Figure 2** – Existing Conditions Site Map. Physical properties of the soil samples are demonstrated by data included in **Appendix A** – Soil Sample Data.

Engineering properties for the foundation and abutment materials were selected from Cone Penetrometer Test (CPT) correlations, field testing, and laboratory testing that supplemented the structural stability and factor of safety assessments for the Bottom Ash Pond. A portion of the engineering properties of the foundation and abutment materials are presented in the "<u>Structural Stability and Safety Factor Assessment Report</u>" (Golder 2016c). Additional engineering properties of the foundation and abutment materials are presented in the "<u>Summary of Monitoring Well Design, Installation, and Development, D.E. Karn Electric Generation Facility</u>" (ARCADIS 2016).

7.0 40 CFR 257.73 (C)(1)(VI)

A statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR unit; the method of site preparation and construction of each zone of the CCR unit; and the approximate dates of construction of each successive stage of construction of the CCR unit.

7.1 Physical and Engineering Properties

Golder sampled and tested the materials that exists in the exterior berm of the Bottom Ash Pond to gather subsurface information and develop certifications for the structural stability and factor of safety assessment. The physical properties are provided in **Appendix A** – Soil Sample Data. A portion of the

Rev. 0 Signed Page 5 of 9

engineering properties of the foundation and abutment materials are presented in the "<u>Structural Stability</u> <u>and Safety Factor Assessment Report</u>" (Golder 2016c). Additional engineering properties of the foundation and abutment materials are presented in the "<u>Summary of Monitoring Well Design, Installation,</u> and Development, D.E. Karn Electric Generation Facility" (ARCADIS 2016).

7.2 Site Preparation and Construction

Site drawings and historical aerial photographs from 1957, 1959 and 1963 included in the PFMA Report (AECOM 2009) were reviewed, and the following sequence of construction was developed:

- Based on a historical photo from 1957 which depicts the J. C. Weadock facility prior to the construction of the DE Karn facility, original dike structures at the facility were constructed at some time after 1957 and before 1959, when the DE Karn facility was placed in service. The ash disposal area was developed by reclaiming land from Saginaw Bay through the construction of a series of breakwater and perimeter dikes and in-filling with miscellaneous fill (AECOM 2009).
- A historical aerial photograph from 1963 depicts a discharge trestle which conveys ash to the area currently occupied by the Bottom Ash Pond (AECOM 2009).
- In a site development specifications drawing from 1985, the area of the current Bottom Ash Pond is identified as the "Karn Bottom Ash Storage Area". It appears that the Bottom Ash Storage Area is bound on the north by a dike and bound on all other sides by the extent of the historic shoreline.

The site drawings contained no details regarding site construction methods for the Bottom Ash Pond, perimeter dike, and associated foundations.

8.0 40 CFR 257.73 (C)(1)(VII)

At a scale that details engineering structures and appurtenances relevant to the design, construction, operation, and maintenance of the CCR unit, detailed dimensional drawings of the CCR unit, including a plan view and cross sections of the length and width of the CCR unit, showing all zones, foundation improvements, drainage provisions, spillways, diversion ditches, outlets, instrument locations, and slope protection, in addition to the normal operating pool surface elevation and the maximum pool surface elevation following peak discharge from the inflow design flood, the expected maximum depth of CCR within the CCR surface impoundment, and any identifiable natural or manmade features that could adversely affect operation of the CCR unit due to malfunction or mis-operation.

Golder developed the following figures, which are attached hereto, for the Bottom Ash Pond at DE Karn:

- Figure 2 Existing Conditions Site Map
- Figure 3 Bottom Ash Pond Characterization Cross Section A-A'

Cross sections were developed based on an EES Survey (May 2016) and subsurface data collected and interpreted by Golder in 2015 and 2016. These cross sections are not intended to illustrate

Rev. 0_Signed Page 6 of 9

a comprehensive conceptual site model representing all data that may be available for the Bottom Ash Pond.

9.0 40 CFR 257.73 (C)(1)(VIII)

A description of the type, purpose, and location of existing instrumentation.

The CCR RCRA Rule requires that a description of the type, purpose, and location of existing instrumentation be provided. Golder included the locations of the known instruments on **Figure 2** – Existing Conditions Site Map.

CEC retained ARCADIS to install RCRA monitoring wells to characterize groundwater quality conditions in the vicinity of the Bottom Ash Pond. The description and location of this existing instrumentation can be found in the "<u>Summary of Monitoring Well Design, Installation, and Development, DE Karn Electric Generation Facility</u>" (ARCADIS 2016).

10.0 40 CFR 257.73 (C)(1)(IX)

Area-capacity curves for the CCR unit.

Area capacity curves for the Bottom Ash Pond were calculated by Golder using survey data collected by EES in May 2016. The area capacity curves are included in the "<u>Inflow Design Flood Control System</u> <u>Plan</u>" completed by Golder for Pond A (Golder 2016b).

11.0 40 CFR 257.73 (C)(1)(X)

A description of each spillway and diversion design features and capacities and calculations used in their determination.

11.1 Spillway and Diversion Description

Based on the "Annual RCRA CCR Surface Impoundment Inspection Report" completed by Golder for the Bottom Ash Pond (Golder 2016), an elevated trestle and pipe system hydraulically conveys sluiced bottom ash to the Bottom Ash Pond. Water is discharged from the pond via one 24-inch steel outflow pipe within the berm into a series of ditches that convey the flow to the NPDES outfall location.

Diversion is provided by the perimeter berm, minimum elevation of 598.00 (NAVD 88) (Golder 2016b), which surrounds the Bottom Ash Pond.

11.2 Capacities and Calculations

Capacities and calculations regarding the spillway and diversion features can be found in Golder's "*Inflow Design Flood Control System Plan*" for the Bottom Ash Pond (2016b).

Rev. 0_Signed Page 7 of 9

12.0 40 CFR 257.73 (C)(1)(XI)

The construction specifications and provisions for surveillance, maintenance, and repair of the CCR unit.

12.1 Construction Specifications

Construction specifications are detailed on drawings included in the PFMA Report (AECOM 2009).

12.2 Surveillance, Maintenance, and Repair

The December 2010 "<u>Coal Ash Landfill Surveillance and Monitoring Program"</u> (SMP) (CEC 2010) outlines CEC's surveillance, maintenance, and repair program specific to each CCR surface impoundment at JH Campbell. Beginning in October 2015, Pond A was inspected by a qualified individual at least weekly and by a qualified professional engineer (QPE) annually in accordance with the CCR RCRA Rule.

13.0 40 CFR 257.73 (C)(1)(XII)

Any record or knowledge of structural instability of the CCR unit.

Weekly inspections of the facility are performed by qualified individuals to detect potentially hazardous conditions or structural weakness per the CCR RCRA Rule and documented internally on CCR Weekly Inspection Observations Forms. Annual inspections at the facility have been performed by AECOM (2009a, 2012), Barr Engineering (2014), and Golder (2016, 2016a).

14.0 ATTACHMENTS

Figure 1 – Site Location Map

Figure 2 – Existing Conditions Site Map

Figure 3 – Bottom Ash Pond Characterization Cross Section A-A'

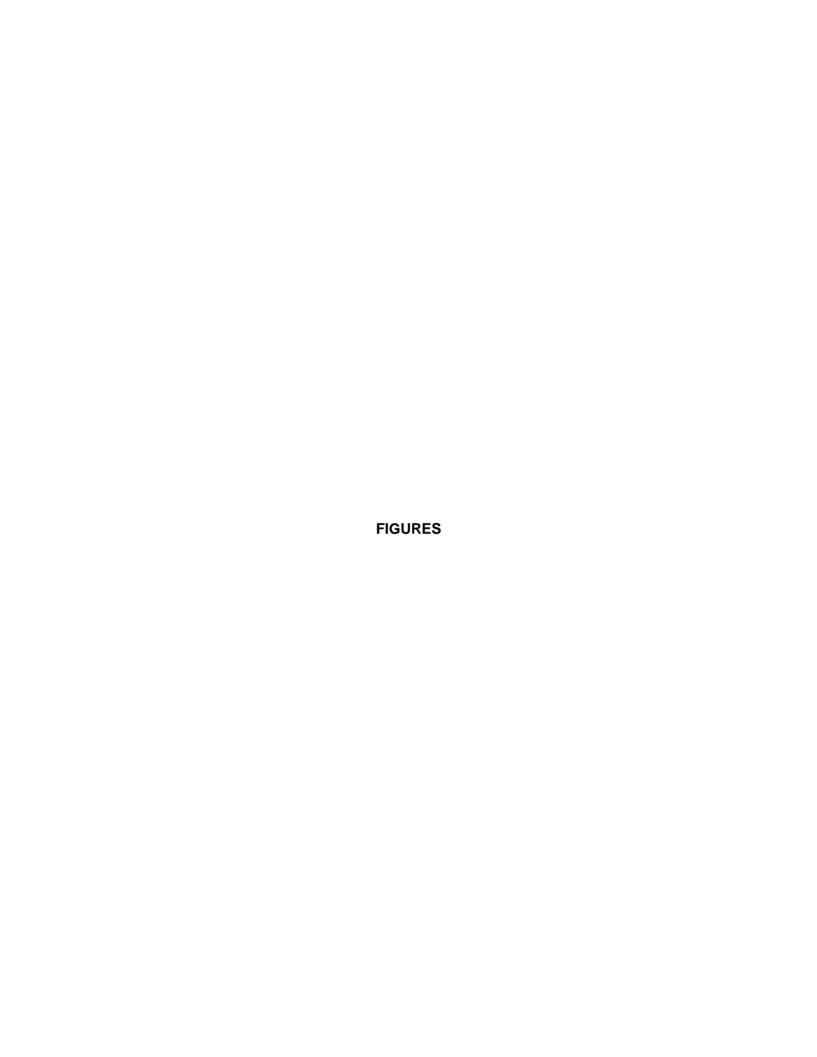
Appendix A - Soil Sample Data

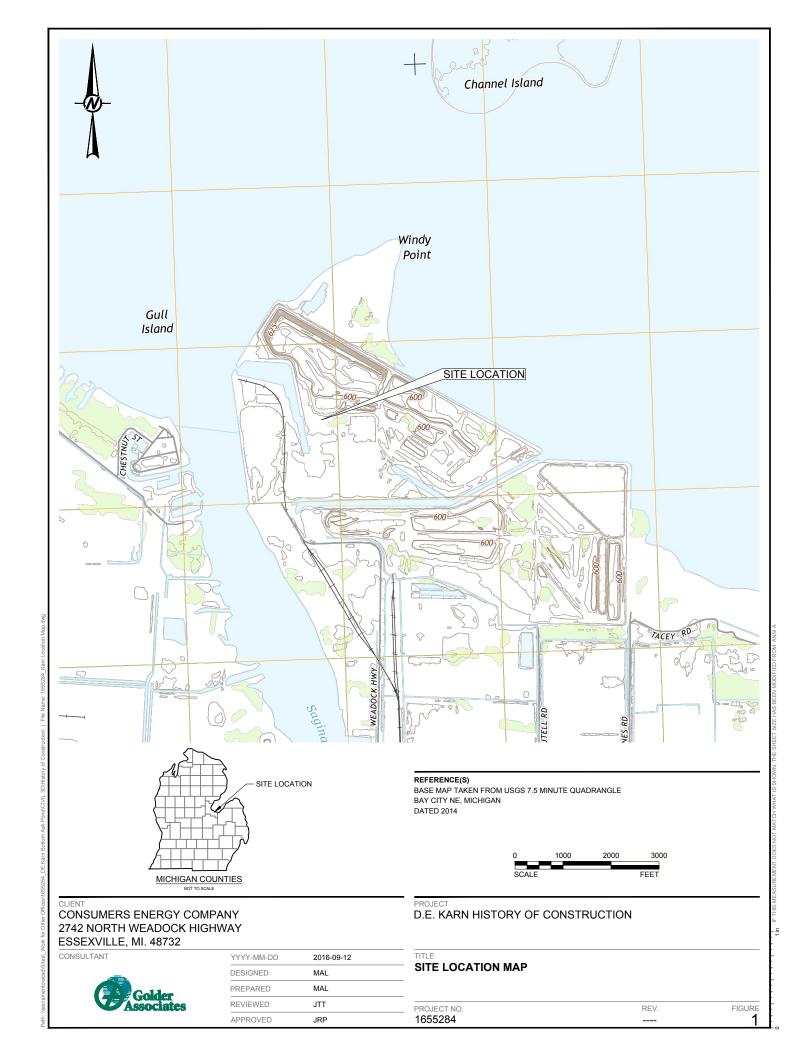
Rev. 0_Signed Page 8 of 9

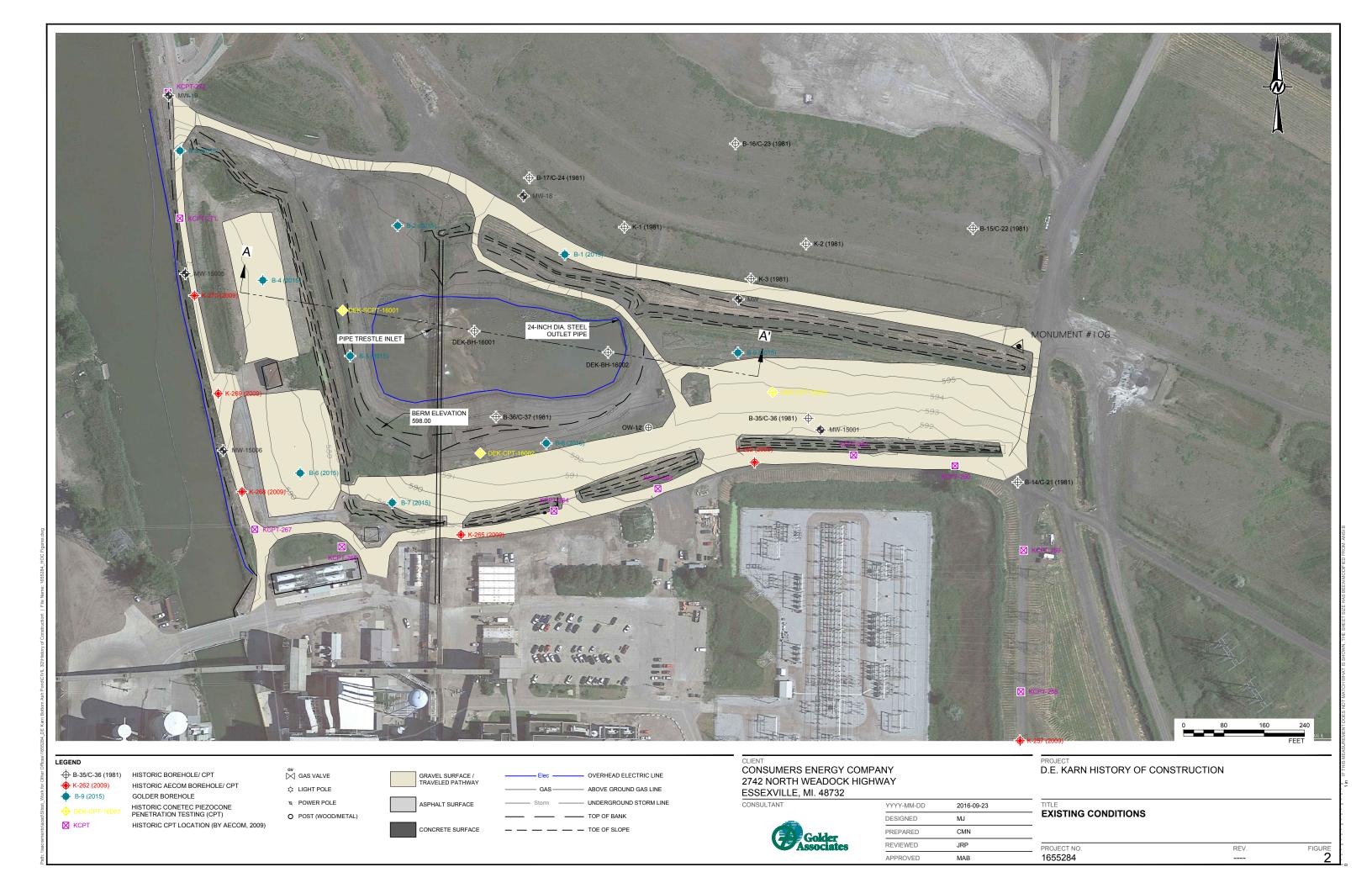
15.0 REFERENCES

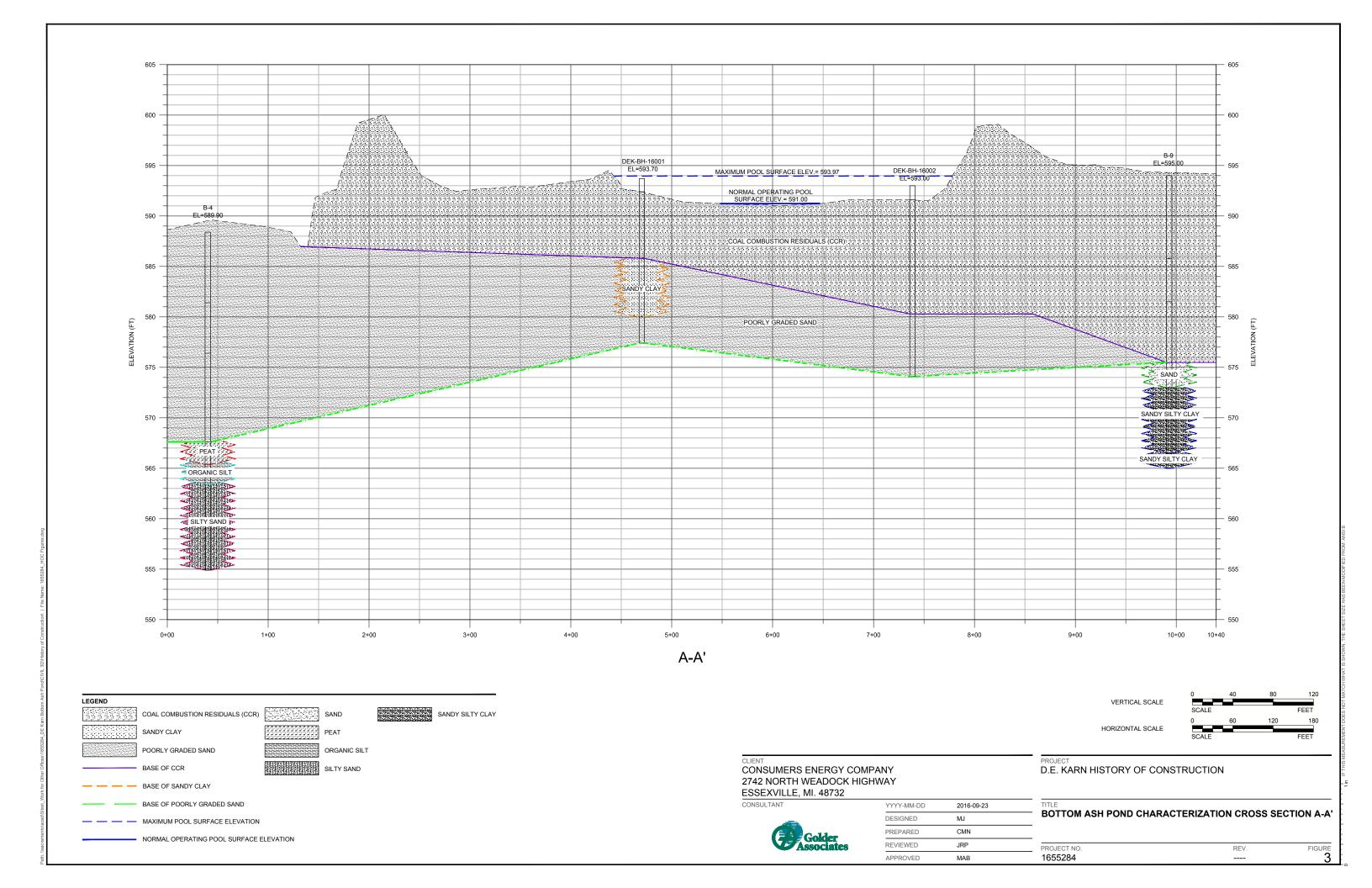
- AECOM (2009). "Potential Failure Mode Analysis (PFMA) Report, D.E. Karn Generating Facility Ash Dike Assessment."
- AECOM (2009a). "Inspection Report D.E. Karn Generating Facility Ash Dike Risk Assessment, Essexville, MI."
- AECOM (2012). "D.E. Karn Ash Disposal Area 2012 Ash Dike Risk Assessment Final Inspection Report."
- ARCADIS (2016). "Summary of Monitoring Well Design, Installation, and Development, D.E. Karn Electric Generation Facility."
- Barr Engineering (2014). "D.E. Karn Generating Facility Triennial Ash Dike Risk Assessment Report Spring 2014."
- Consumers Energy (2010). "Coal Ash Landfill Surveillance and Monitoring Program."
- Golder Associates (2016). "<u>D.E. Karn Bottom Ash Pond, Annual RCRA CCR Surface Impoundment Inspection Report January 2016.</u>"
- Golder Associates (2016a). "D.E. Karn Bottom Ash Pond Annual Inspection Report."
- Golder Associates (2016b). "D.E. Karn Generating Facility Bottom Ash Pond Inflow Design Flood Control System Plan."
- Golder Associates (2016c). "D.E. Karn Generating Facility Bottom Ash Pond Structural Stability and Safety Factor Assessment Report."
- USEPA 40 CFR Parts 257 and 261; Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities, (2015). Environmental Protection Agency, Washington D.C. epa.gov.
- USEPA MyWATERS Mapper (2016). https://watersgeo.epa.gov/mwm.

Rev. 0_Signed Page 9 of 9



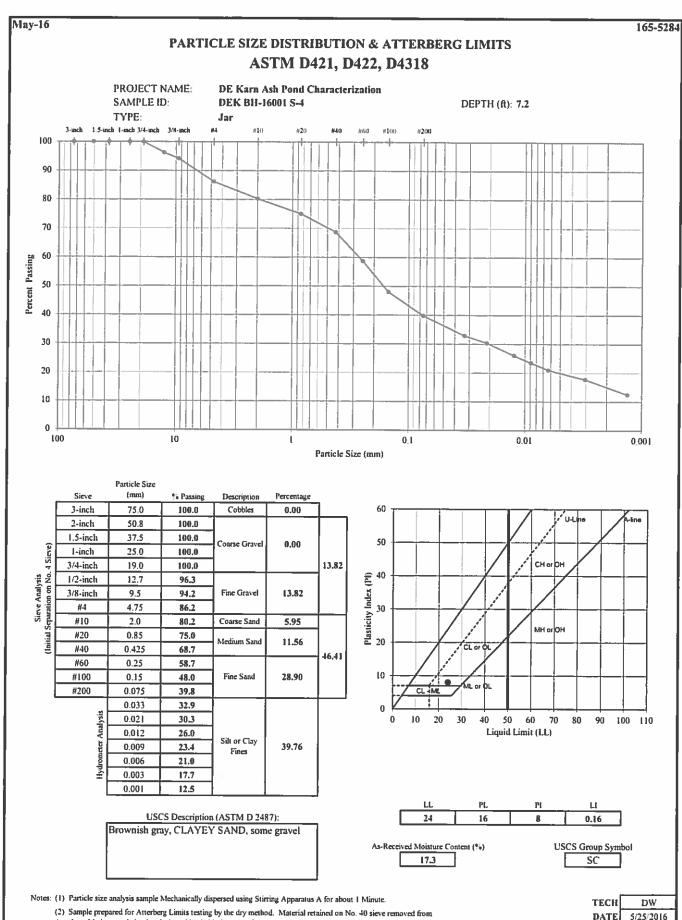






APPENDIX A SOIL SAMPLE DATA





Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed

using manual device.

DATE

CHECK

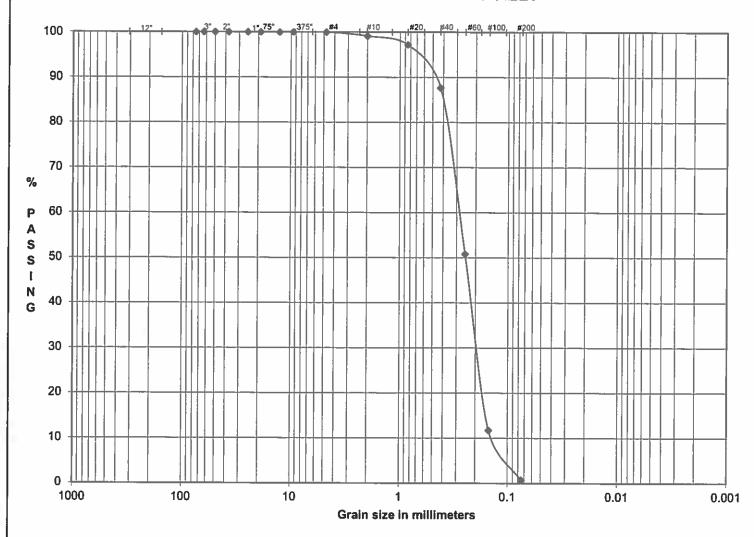
REVIEW

5/25/2016

ASTM GRAIN SIZE ANALYSIS ASTM D 421, D 2217, D 1140, C 117, D 422, C 136, C 142

PROJECT TITLE	D.E	. Karn Ash Pon	d Characteri	zation		AMPLE ID	DEK-BH-	16001 S-5
PROJECT NO.		165-5284		_	SAM	PLE TYPE	J	ar
REMARKS						DEPTH (ft)	1;	3.5
				Hygroscopic N	Moisture For S	ieve Sample		
WATER CONTENT (De	elivered Moist			_		Wet Soil & 7	Γare (gm)	1.00
Wt Wet Soil & Tare (gm)		(w1) <u>34.66</u>		_		Dry Soil & T	are (gm)	1.00
Wt Dry Soil & Tare (gm)		(w2)	31.26			Tare Weight		0.00
Weight of Tare (gm)		(w3)	14.03			Moisture Co		0.00%
Weight of Water (gm)		(w4=w1-w2)	3.40	Total Weight	Of Sample Use	ed For Sieve Co	rrected For Hyg	roscopic Moistur
Weight of Dry Soil (gm)		(w5=w2-w3)	17.23	1		Weight Of S		618.95
Moisture Content (%)		(w4/w5)*100	19.73	_		Tare Weight	(gm)	317.89
				<u></u>	(W6)	Total Dry W	eight (gm)	301.06
SIEVE ANALYSIS			Cum. Ret.	Cumulative				
Tare Weight		Wt Ret	(Wt-Tare)	(%Retained)	% PASS	SIEVE		
317.89]	+Tare	(dry)	((wt ret/w6)*100)	(100-%ret)			
· · ·	-				,,			
	3.0"	317.89	0.00	0.00	100,00	3.0"	coarse gravel	
	2.5"	317.89	0.00	0.00	100.00	2.5"	coarse gravel	
	2.0"	317.89	0.00	0.00	100.00	2.0 ^H	coarse gravel	
	1.5"	317.89	0.00	0.00	100.00	1.5"	coarse gravel	
	1.0"	317.89	0.00	0.00	100.00	1.0"	coarse gravel	
	0.75"	317.89	0.00	0.00	100,00	0.75"	fine gravel	
	0.50"	317.89	0.00	0.00	100.00	0.50"	fine gravel	
	0.375"	317.89	0.00	0,00	100.00	0.375"	fine gravel	
	#4	318.06	0.17	0.06	99.94	#4	coarse sand	
	#10	320.47	2.58	0.86	99.14	#10	medium sand	
	#20	326.32	8.43	2.80	97.20	#20	medium sand	
	#40	355.05	37.16	12.34	87.66	#40	fine sand	
	#60	466.00	148.11	49.20	50.80	#60	fine sand	
	#100	583.53	265.64	88.23	11.77	#100	fine sand	
	#200	616.78	298.89	99.28	0.72	#200	fines	
				-				
				-			·	
% C GRAVEL	0.00	Descript	ive Terms	> 10% mo	stly coarse (c)			
% F GRAVEL	0.06	trace	0 to 5%	> 10% mo	stly medium (r	n)	LL	
% C SAND	0.80	little	5 to 12%	< 10% fin	e (c-m)		PL	-
% M SAND	11.49	some	12 to 30%	< 10% coa	ırse (m-f)		PI	-
% F SAND	86.94	and	30 to 50%	< 10% coa	irse and fine (n	n)	Gs	-
% FINES	0.72	1			rse and mediu			
% TOTAL	100.00	J		> 10% equ	ial amounts ea	ch (c-f)		
DE	SCRIPTION	Brown, POORI	Y GRADED	SAND, trace gr	avel, trace			
		fines						
							_	
	USCS	SP					ТЕСН	DF
							DATE	5/26/2016
							CHECK	F2-5
	* material finer	than #4 sieve cor	rected for hygr	oscopic moisture.			REVIEW	13517

PARTICLE SIZE DISTRIBUTION ASTM D 421 AND D 422 US STANDARD SIEVE OPENING SIZES



		Coarse	Fine	Cor	Med	Fine	SILT OR CLAY
Boulders	Cobbles	GRAV	ÆL_		SAND		FINES
`	0,00	0.00	0.06	0.80	11.49	86.94	0.72
		0.0	6		99	22	

SAMPLE ID	DEK-BH-16001 S-5
SAMPLE TYPE	
SAMPLE DEPTH (ft)	13.5

LL	-
PL	-
ΡI	-

DESCRIPTION	Brown, POOF	RLY GRADED SAND, trace gravel, trace fines
USCS	SP	

TECH	DF
DATE	5/26/2016
CHECK	125
REVIEW	PST



May-16

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318

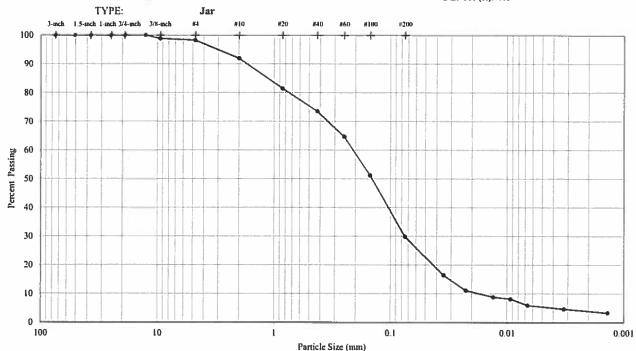
PROJECT NAME: SAMPLE ID:

DE Karn Ash Pond Characterization

DEK BH-16002 S-2

DEPTH (ft): 7.0

165-5284



	Sieve	Particle Size (mm)	% Passing	Description	Percentage	
	3-inch	75.0	100.0	Cobbles	00,0]
	2-inch	50.8	100.0			
	1.5-inch	37.5	100.0	Coarse Gravel	0.00	l
Sieve)	l-inch	25.0	100.0	Coarse Oravei	0.00	l
- S	3/4-inch	19.0	100.0	1		1.82
No N	1/2-inch	12.7	100.0		·	1
n on	3/8-inch	9.5	98.8	Fine Gravel	1.82	
Sieve Analysis paration on No	#4	4.75	98.2]		
Sieve Al (Initial Separation	#10	2.0	91.9	Coarse Sand	6.29	
iat	#20	0.85	81.4	Medium Sand	18.43	1
E,	#40	0.425	73.5	arcanin Sand	10.43	68.34
	1160	0.25				Too⊸>+

#60 64.6 Fine Sand 43.62 #100 0.15 51.2 #200 0.075 29.8 0.035 16.5 0.023 11.1 0.013 8.8 Silt or Clay 29.84 0.009 8.2 Fines 0.007 5.9 0.003 4.6 100,0 3.3

60 50 CH or OH Plasticity Index (Pl) 30 MH or OH 10 CL IML 0 30 40 50 60 lO 20 90 100 110 Liquid Limit (LL)

Visual Description: Very dark gray, CCR, trace gravel

As-Received Moisture Content (%) 69.7

USCS Group Symbol

Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

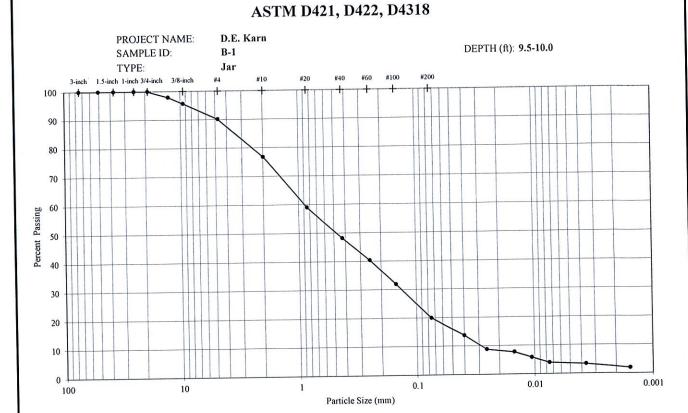
DW TECH 5/24/2016 DATE CHECK REVIEW



March-15

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

PARTICLE SIZE DISTRIBUTION & PARTICLE SIZE DIS



	Percentage	Description	% Passing	Particle Size (mm)	Sieve	
	0.00	Cobbles	100.0	75.0	3-inch	
			100.0	50.8	2-inch	
	0.00	Coarse Gravel	100.0	37.5	1.5-inch	
	0,00	Course Grave.	100.0	25.0	1-inch	
9.85			100.0	19.0	3/4-inch	
			97.9	12.7	1/2-inch	
	9.85	Fine Gravel	95.7	9.5	3/8-inch	
			90.2	4.75	#4	
1	13.36	Coarse Sand	76.8	2.0	#10	
	28.59	Medium Sand	59.0	0.85	#20	
70.16		Wicalam Came	48.2	0.425	#40	
			40.4	0.25	#60	
	28.21	Fine Sand 28.21	Fine Sand	31.9	0.15	#100
			20.0	0.075	#200	
			13.7	0.040		
			8.8	0.026	lysis	
		Silt or Clay	7.8	0.015	Ana	
	20.00	Fines	5.9	0.011	Hydrometer Analysis	
			4.0	0.008	rom	
]	3.4	0.004	Hy	
			2.0	0.002		

Dorticle Size

50			CH o			
40			/ CHO			
30						
20		CL or OL	МН	or OH		
10		ML or OL				
0	10 20		0 60	70 80	90	100 110
v	10 20	Liqui	d Limit (Ll			

Visual Description:
Very dark gray, BOTTOM ASH

LL PL PI LI
-- -- -- --

As-Received Moisture Content (%)
16.2

USCS Group Symbol

Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

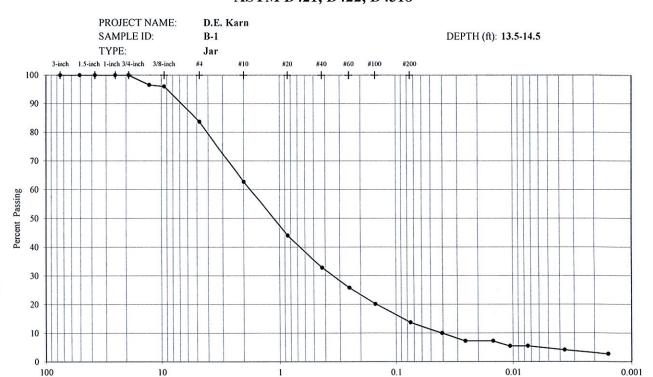
(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

TECH HD
DATE 3/16/2015
CHECK
REVIEW





PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318



Particle Size (mm)

	Sieve	Particle Size (mm)	% Passing	Description	Percentage	
	3-inch	75.0	100.0	Cobbles	0.00	1
	2-inch	50.8	100.0			
	1.5-inch	37.5	100.0	Coarse Gravel	0.00	
4 Sieve)	1-inch	25.0	100.0	Coarse Graver	0.00	
4 Si	3/4-inch	19.0	100.0	1		16.34
No.	1/2-inch	12.7	96.6			1
lan o	3/8-inch	9.5	96,1	Fine Gravel	16.34	
Sieve Analysis paration on No	#4	4.75	83.7			
Sieve Analysis (Initial Separation on No.	#10	2.0	62.7	Coarse Sand	21.00	
	#20	0.85	44.0	Medium Sand	29.83	
(Init	#40	0.425	32.8	Wedium Sand	29.83	69.92
	#60	0.25	25.8		19.09	09.92
	#100	0.15	20.2	Fine Sand		
1	#200	0.075	13.7			
•		0.041	10.0			
	Hydrometer Analysis	0.026	7.3	1		
	Anal	0.015	7.3]		
	eter	0.011	5.5	Silt or Clay Fines	13.74	
	rom	0.007	5.5]		
	Hyd	0.004	4.1			
		0.002	2.8			

50			/	/	U-Line		A-line
40				CHorO			
30							
20		/ CL or OL	1	MH or O	H		
10	CL ML	ML or OL					
0	10 20	30 40	50 quid Lim	60 70	80	90 1	00

Visual Description:
Very dark gray, BOTTOM ASH

LL PL PI LI

As-Received Moisture Content (%)
23.7

USCS Group Symbol

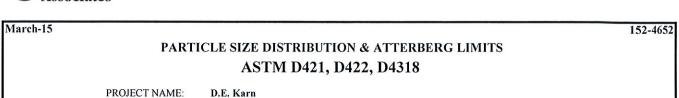
Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

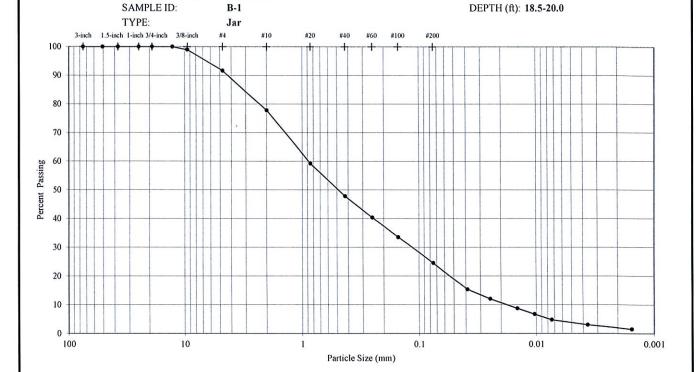
(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

TECH HD

DATE 3/16/2015
CHECK
REVIEW







60

50

Plasticity Index (PI) 30 00

	Particle Size			
Sieve	(mm)	% Passing	Description	Percentage
3-inch	75.0	100.0	Cobbles	0.00
2-inch	50.8	100.0		
1.5-inch	37.5	100.0	Coarse Gravel	0.00

2-inch 1.5-inch 1.5-inch 3/4-inch 3/4-inch 3/8-inch 3/8-inch 3/8-inch 4/4 4/10 4/60 4/100 4/200	2-inch	50.8	100.0			
	1.5-inch	37.5	100.0	Coarse Gravel	0.00	
eve [1-inch	25.0	100.0	Coarse Graver	0.00	
4 Si	3/4-inch	19.0	100.0			8.37
SIS OO	1/2-inch	12.7	100.0			1
la la	3/8-inch	9.5	98.9	Fine Gravel	8.37	
ation	#4	4.75	91.6	1		
epar se	#10	2.0	77.8	Coarse Sand	13.85	
ial S	#20	0.85	59.2	Medium Sand	20.02	1
(Init	#40	0.425	47.8	Wedium Sand	30.02	67.00
	#60	0.25	40.3			7 67.03
[#100	0.15	33.5	Fine Sand	23.23	
[#200	h 25.0 ch 19.0 ch 12.7 ch 9.5 4.75 2.0 0.85 0.425 0.25 0.15	24.5			
		0.039	15.4			67.09
	lysis	0.025	12.1	7	8.37 13.85 30.02	
	Ana	0.015	8.8	T		
1.5- 3	eter	0.010	6.8	Silt or Clay Fines 24	24.54	1
	rom	0.007	4.8	165		
	Py Py	0.004	3.1	7		

0.002

	10	CL	ML	ML	or OL					
	0 1	10	20	30	40 Li	50 quid L	60 imit (I	70 .L)	80	90
		782						2		
	1	- 1	LL		PL		P			LI
_							-			

Visual Description:
Very dark grayish brown, BOTTOM ASH

As-Received Moisture Content (%)
24.6

USCS Group Symbol

CH or OH

MH or OH

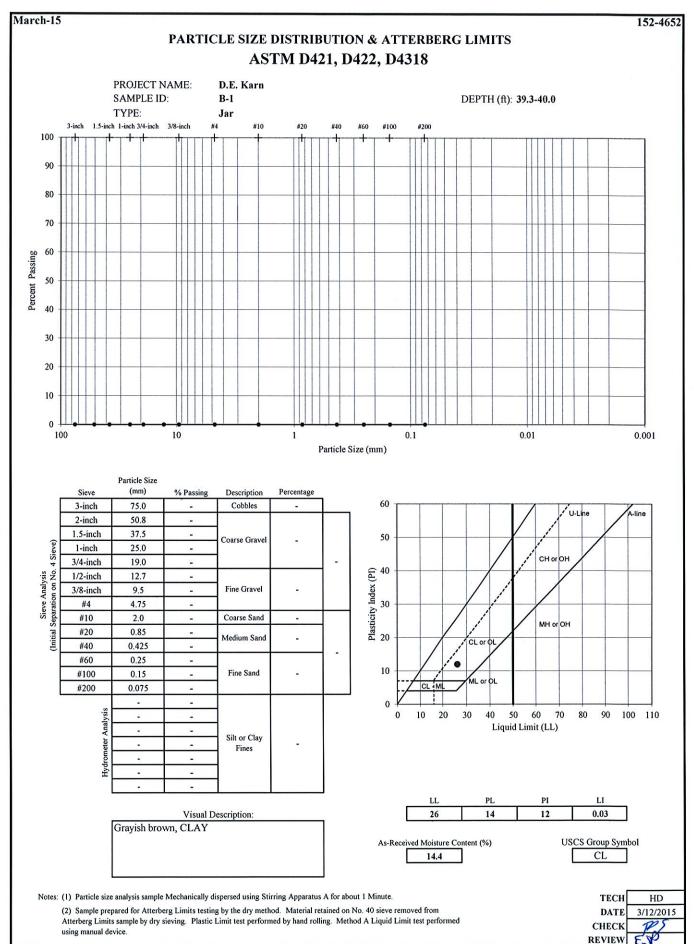
Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

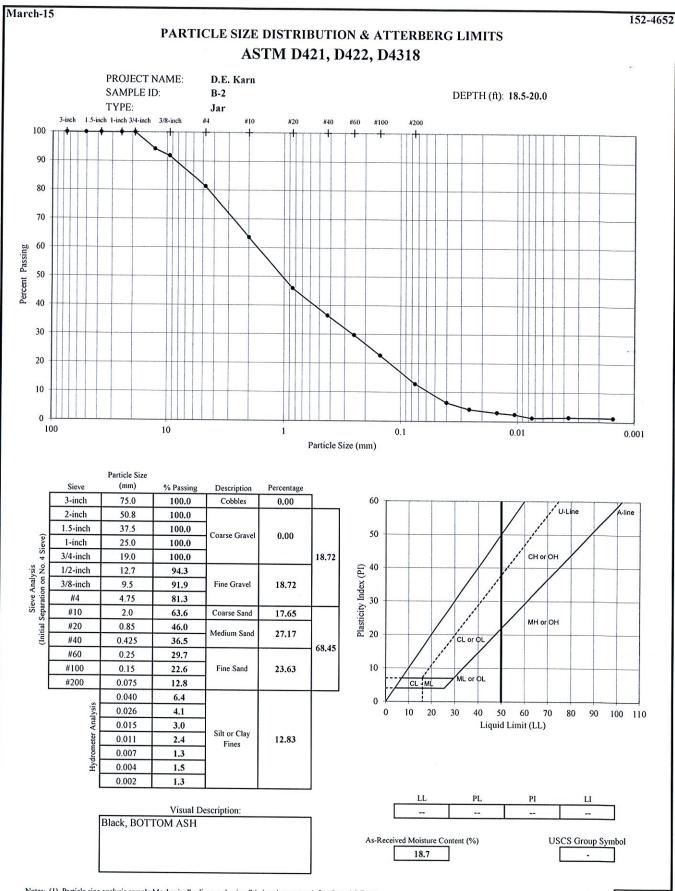
TECH HD
DATE 3/9/2015
CHECK PREVIEW

100 110





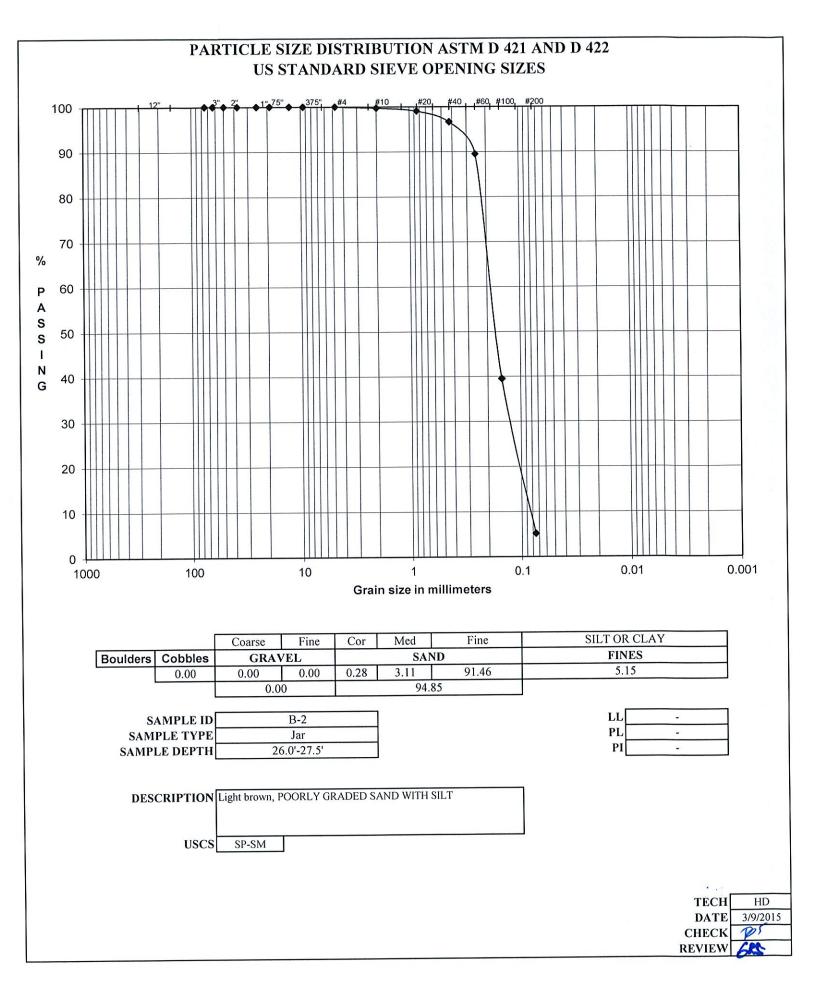




Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

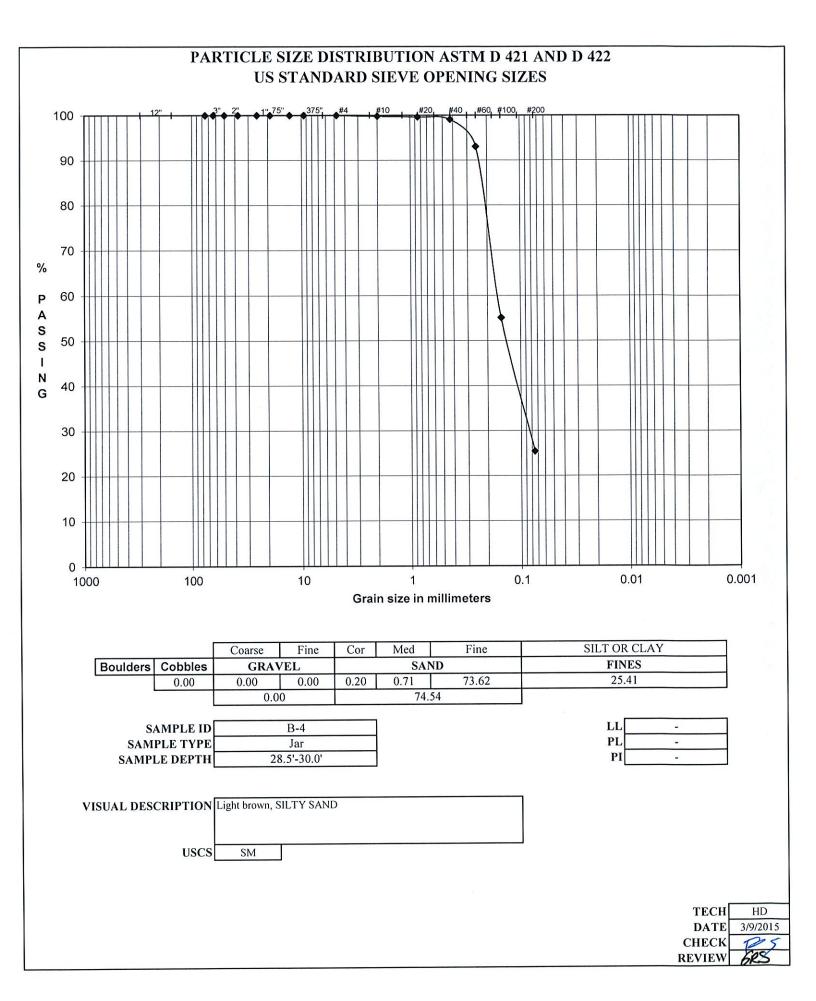
(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device. TECH HD
DATE 3/9/2015
CHECK P5
REVIEW 5

ASTM GRAIN SIZE ANALYSIS ASTM D 421, D 2217, D 1140, C 117, D 422, C 136, C 142 B-2 SAMPLE ID D.E. Karn Ash Tank PROJECT TITLE SAMPLE TYPE Jar 152-4652 PROJECT NO. 26.0'-27.5' SAMPLE DEPTH REMARKS Hygroscopic Moisture For Sieve Sample 1.00 Wet Soil & Tare (gm) WATER CONTENT (Delivered Moisture) 1.00 Dry Soil & Tare (gm) 336.71 (w1) Wt Wet Soil & Tare (gm) 0.00 Tare Weight (gm) 280.90 (w2)Wt Dry Soil & Tare (gm) 0.00% Moisture Content (%) 52.05 (w3)Weight of Tare (gm) Total Weight Of Sample Used For Sieve Corrected For Hygroscopic Moisture (w4=w1-w2)55.81 Weight of Water (gm) 280.90 Weight Of Sample (gm) 228.85 (w5=w2-w3)Weight of Dry Soil (gm) 52.05 Tare Weight (gm) 24.39 (w4/w5)*100 Moisture Content (%) 228.85 (W6) Total Dry Weight (gm) Cumulative Cum. Ret. SIEVE ANALYSIS SIEVE (%Retained) % PASS Wt Ret (Wt-Tare) Tare Weight (100-%ret) +Tare (dry) {(wt ret/w6)*100} 52.06 0.00 100.00 3.0" coarse gravel 0.00 52.06 3.0" coarse gravel 100.00 2.5" 52.06 0.00 2.5" 0.00 100.00 2.0" coarse gravel 0.00 0.00 52.06 2.0" coarse gravel 0.00 100.00 1.5" 0.00 52.06 1.5" 100.00 1.0" coarse gravel 0.00 0.00 1.0" 52.06 0.75" fine gravel 0.00 100.00 52.06 0.00 0.75" fine gravel 0.00 0.00 100.00 0.50" 52.06 0.50" 100.00 0.375" fine gravel 0.00 0.00 52.06 0.375" coarse sand 0.00 100.00 #4 52.06 0.00 #4 medium sand 0.65 0.28 99.72 #10 52.71 #10 99.05 #20 medium sand 0.95 54.24 2.18 #20 fine sand 96.61 #40 3.39 59.82 7.76 #40 fine sand 89.46 #60 24.12 10.54 76.18 #60 fine sand 39.47 #100 60.53 190.58 138.52 #100 fines #200 94.85 5.15 217.07 #200 269.13 > 10% mostly coarse (c) Descriptive Terms 0.00 % C GRAVEL LL > 10% mostly medium (m) 0 to 5% 0.00 trace % F GRAVEL PL < 10% fine (c-m) 5 to 12% 0.28 little % C SAND PΙ < 10% coarse (m-f) 12 to 30% 3.11 some % M SAND Gs < 10% coarse and fine (m) and 30 to 50% 91.46 % F SAND < 10% coarse and medium (f) 5.15 % FINES > 10% equal amounts each (c-f) 100.00 % TOTAL Light brown, POORLY GRADED SAND WITH SILT DESCRIPTION HD **TECH USCS** SP-SM DATE 3/9/2015 CHECK REVIEW * material finer than #4 sieve corrected for hygroscopic moisture.

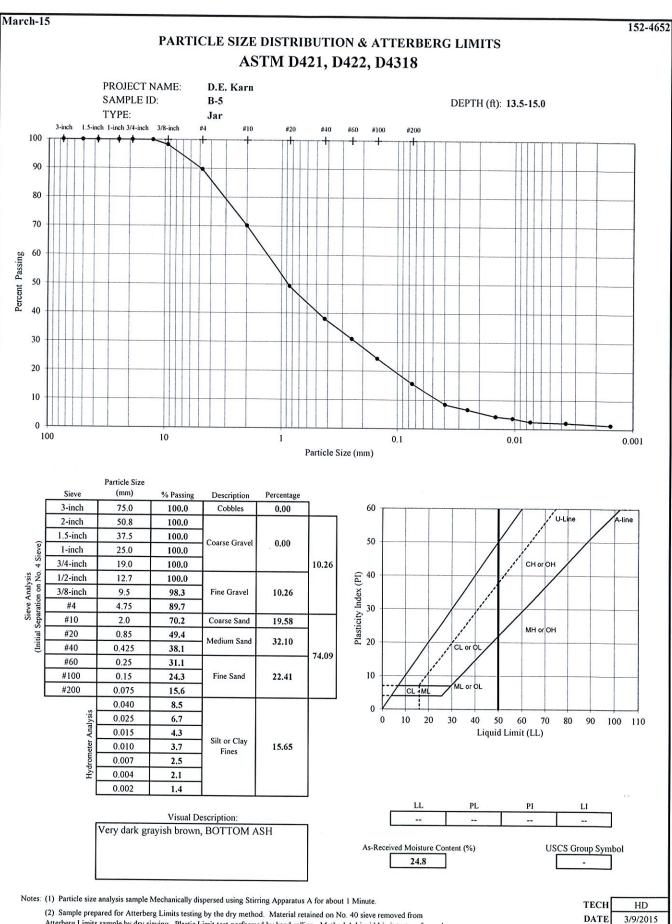


ASTM GRAIN SIZE ANALYSIS ASTM D 421, D 2217, D 1140, C 117, D 422, C 136, C 142

PROJECT TITLE		.E. Karn Ash Tank			SAMPLE ID		-4	
PROJECT NO.		152-4652				PLE TYPE		
REMARKS						LE DEPTH	28.5'-	-30.0'
				Hygroscopic N	Moisture For Sieve Sample			
WATER CONTENT (De		The second secon		Wet Soil & Tare (gm)				1.00
Wt Wet Soil & Tare (gm)		(w1)	292.73			Dry Soil & T		1.00
Wt Dry Soil & Tare (gm)		(w2)	252.17	1		Tare Weight		0.00
Weight of Tare (gm)		(w3)	51.65			Moisture Co		0.00%
Weight of Water (gm)		(w4=w1-w2)	40.56	Total Weight (Of Sample Use	d For Sieve C	orrected For Hygi	roscopic Moisture
Weight of Dry Soil (gm)		(w5=w2-w3)	200.52]		Weight Of S	ample (gm)	252.17
Moisture Content (%)		(w4/w5)*100	20.23]		Tare Weigh	t (gm)	51.65
					(W6)	Total Dry W	eight (gm)	200.52
SIEVE ANALYSIS			Cum. Ret.	Cumulative				
Tare Weight	-	Wt Ret	(Wt-Tare)	(%Retained)	% PASS	SIEVE		
51.57		+Tare	(dry)	{(wt ret/w6)*100}	(100-%ret)			
	3.0"	51.67	0.10	0.05	99.95	3.0"	coarea aroxal	
	2.5"	51.67	0.10	0.05	99.95	2.5"	coarse gravel	
	2.0"	51.67	0.10	0.05	99.95	2.0"	coarse gravel	
	1.5"	51.67	0.10	0.05	99.95	1.5"	coarse gravel	
	1.0"	51.67	0.10	0.05	99.95	1.0"	coarse gravel	
	0.75"	51.67	0.10	0.05	99.95	0.75"	fine gravel	
	0.75	51.67	0.10	0.05	99.95	0.75	fine gravel	
	0.375"	51.67	0.10	0.05	99.95	0.375"	fine gravel	
	#4	51.67	0.10	0.05	99.95	#4	coarse sand	
	#10	52.08	0.10	0.05	99.75	#10	medium sand	
	#20	52.42	0.85	0.42	99.58	#20	medium sand	
	#40	53.50	1.93	0.42	99.04	#40	fine sand	
	#60	65.61	14.04	7.00	93.00	#60	fine sand	
	#100	141.68	90.11	44.94	55.06	#100	fine sand	
	#200	201.13	149.56	74.59	25.41	#200	fines	
	#200	201.13	147.50	74.57	23.41	11200	inies	
		_						
% C GRAVEL	0.00	Descript	tive Terms	> 10% mg	ostly coarse (c)			
% F GRAVEL	0.00	trace	0 to 5%	> 10% me	ostly medium (m)	LL	-
% C SAND	0.20	little	5 to 12%	< 10% fir	ie (c-m)		PL	-
% M SAND	0.71	some	12 to 30%	< 10% co	arse (m-f)		PI	-
% F SAND	73.62	and	30 to 50%	< 10% co	arse and fine (n)	Gs	-
% FINES	25.41			< 10% co	arse and mediu	ım (f)		
% TOTAL	99.95	_		> 10% eq	ual amounts ea	ich (c-f)		
WORK T	SCODIESTO:	Links become O	H TV CAND			1		
VISDUAL DE	ESCRIPTION	Light brown, S	ILIY SAND					
	USCS	SM					TECH	HD
							DATE	3/9/2015
							CHECK	TOS
	* material fine	r than #4 sieve co	rrected for hyp	rosconic moistur	9		REVIEW	GRS







(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device. TECH HD
DATE 3/9/2015
CHECK REVIEW



March-15 152-4652 PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318 PROJECT NAME: D.E. Karn SAMPLE ID: B-5 DEPTH (ft): 18.5-20.0 TYPE: Jar 3-inch 1.5-inch 1-inch 3/4-inch 3/8-inch #10 #20 #60 #100 100 90 80 70 60 Percent Passing 40 30 20 10 0 10 100 1 0.1 0.01 0.001 Particle Size (mm) Particle Size (mm) % Passing Percentage Sieve Description 60 3-inch 75.0 100.0 Cobbles 0.00 U-Line A-line 2-inch 50.8 100.0 1.5-inch 37.5 100.0 50 Coarse Gravel 0.00 Sieve Analysis (Initial Separation on No. 4 Sieve) 1-inch 25.0 100.0 0.00 3/4-inch 19.0 100.0 Plasticity Index (PI) 30 1/2-inch 12.7 100.0 Fine Gravel 3/8-inch 9.5 100.0 0.00 #4 4.75 100.0 #10 99.4 0.55 2.0 Coarse Sand MH or OH 98.2 #20 0.85 Medium Sand 2.01 #40 0.425 97.4 16.84 #60 0.25 96.0 10 Fine Sand 14.28 #100 0.15 92.4 #200 0.075 83.2 0.032 69.8 0 10 20 30 80 90 0.022 40 50 60 70 100 110 57.4 Liquid Limit (LL) 0.012 37.4 Silt or Clay 0.010 83.16 29.7 0.007 20.3 0.004 8.2 0.002 3.2 PL Visual Description: Dark gray, FLY ASH USCS Group Symbol As-Received Moisture Content (%) 58.4

Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

TECH HD DATE 3/9/2015 CHECK REVIEW



March-15 152-4652 PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS (WET) ASTM D421, D422, D4318 PROJECT NAME: D.E. Karn DEPTH (ft): 33.5-35.0 SAMPLE ID: B-5 Bag TYPE: 3-inch 1.5-inch 1-inch 3/4-inch 3/8-inch #10 100 90 80 70 60 Percent Passing 50 40 30 20 10 100 10 1 0.1 0.01 0.001 Particle Size (mm) Particle Size (mm) Description Percentage Sieve % Passing 90 75.0 Cobbles 3-inch A-line U-Line 50.8 2-inch 80 1.5-inch 37.5 Coarse Gravel 25.0 1-inch 70 **С**Н ог ОН 3/4-inch 19.0 Index (PI) 20 Sieve Analysis Separation on No. 4 1/2-inch 12.7 3/8-inch 9.5 Fine Gravel 4.75 Plasticity I #10 2.0 Coarse Sand MH or OH #20 0.85 Medium Sand 0.425 #40 Ck or OL 0.25 20 #60 #100 0.15 Fine Sand 10 #200 0.075 ML or OL 0 Hydrometer Analysis 0 40 80 100 120 140 Liquid Limit (LL) Silt or Clay Fines USCS Description (ASTM D 2487): 134 0.76

As-Received Moisture Content (%)

117.5

Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

Very dark gray, PEAT

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

TECH HD

DATE 3/12/2015
CHECK
REVIEW EST

USCS Group Symbol

PT



March-15 152-4652

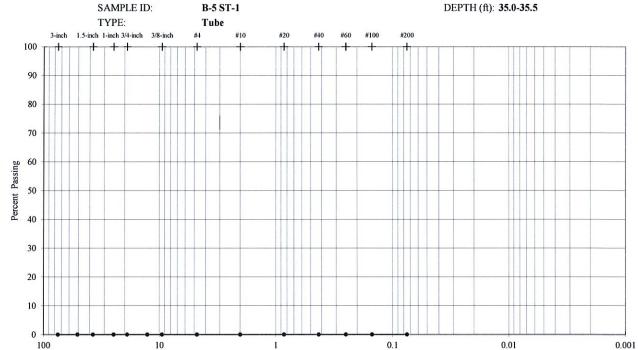
PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS (WET) ASTM D421, D422, D4318

PROJECT NAME:

D.E. Karn

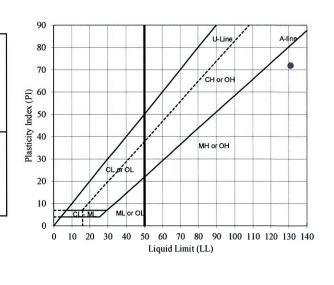
B-5 ST-1

DEPTH (ft): 35.0-35.5



Particle Size (mm)

	Sieve	Particle Size (mm)	% Passing	Description	Percentage		
Γ	3-inch	75.0	-	Cobbles	-	1	
Ī	2-inch	50.8	-				
	1.5-inch	37.5	-	Coarse Gravel			
4 Sieve)	1-inch	25.0	-	Coarse Graver	-		
4 Si	3/4-inch	19.0	-			-	
Sieve Analysis (Initial Separation on No.	1/2-inch	12.7	-				
Sieve Analysis paration on No	3/8-inch	9.5	-	Fine Gravel	-		
ve /	#4	4.75	-				
Sie	#10	2.0	-	Coarse Sand			
ial S	#20	0.85	-	Medium Sand			
(Ini	#40	0.425	-	Wicdidin Sand	100		
	#60	0.25	-]	
	#100	0.15	-	Fine Sand	-		
	#200	0.075	-				
		-	(a)				
	lysis		-				
	Ana	-	-	671 61			
	eter	(2)	-	Silt or Clay Fines	-		
	Hydrometer Analysis		-	3.500			
	Hyd	-	-				
	2323	-	-				



USCS Description (ASTM D 2487): Black, PEAT

131 0.40

As-Received Moisture Content (%) 87.7

USCS Group Symbol PT

Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

TECH 3/13/2015 DATE CHECK REVIEW

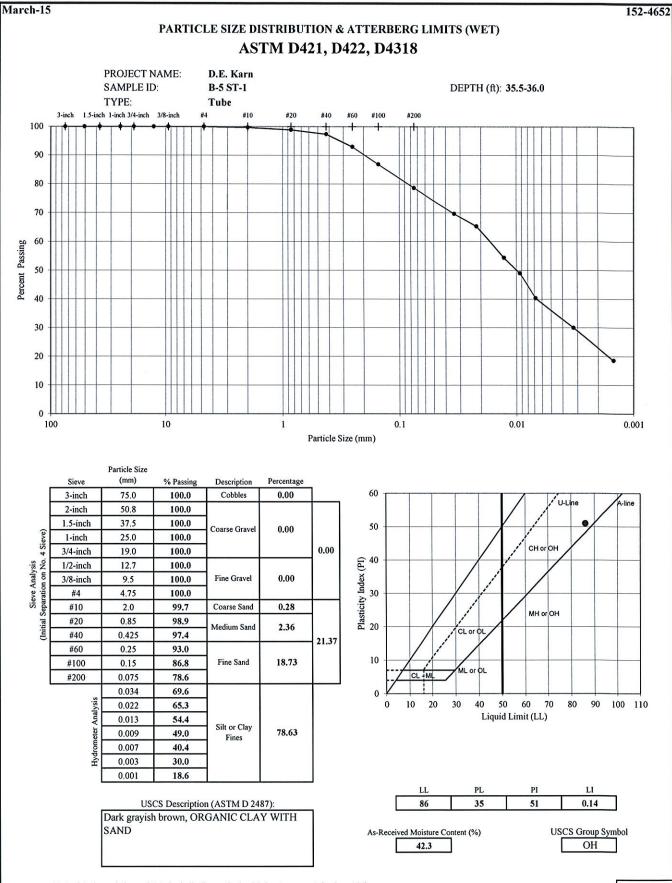
SPECIFIC GRAVITY OF SOIL SOLIDS

ASTM D 854

PROJECT NAME: D.E. Karn PROJECT NUMBER: 152-4652 SAMPLE ID: B-5 ST-1

SAMPLE ID: B-5 ST-1 SAMPLE TYPE: Tube	SAMPLE DEPTH (ft): 35.0-35.5								
SAMPLE PREPARATION									
Wet (A) or Dry (B) B	% Passing #4 Sieve #DIV/0!								
SAMPLE DESCRIPTION Black, PEAT	NOTES								
CALCULA	TIONS								
Test Temperature, T _t	18.5 °C								
Density of Water @ T _t	0.99850 g/mL								
Pycnometer Number Mass of Pycnometer, Water, & Soil (M _{pws} @ T _t) Mass of Pycnometer & Water (M _{pw} @ T _t)	10 383.98 g 362.75 g								
Mass of Dish & Dry Soil Mass of Dish Mass of Oven Dry Soil (M _s)	404.43 g 367.99 g 36.44 g								
Specific Gravity (G _s @ T _t)	2.40								
Temperature coefficient (K)	1.00030								
Specific Gravity (G _s @ 20 °C)	2.40								
Golder Associates Inc. Lansing, Michigan	TECH HD DATE 3/13/2015 CHECK REVIEW F. NO								





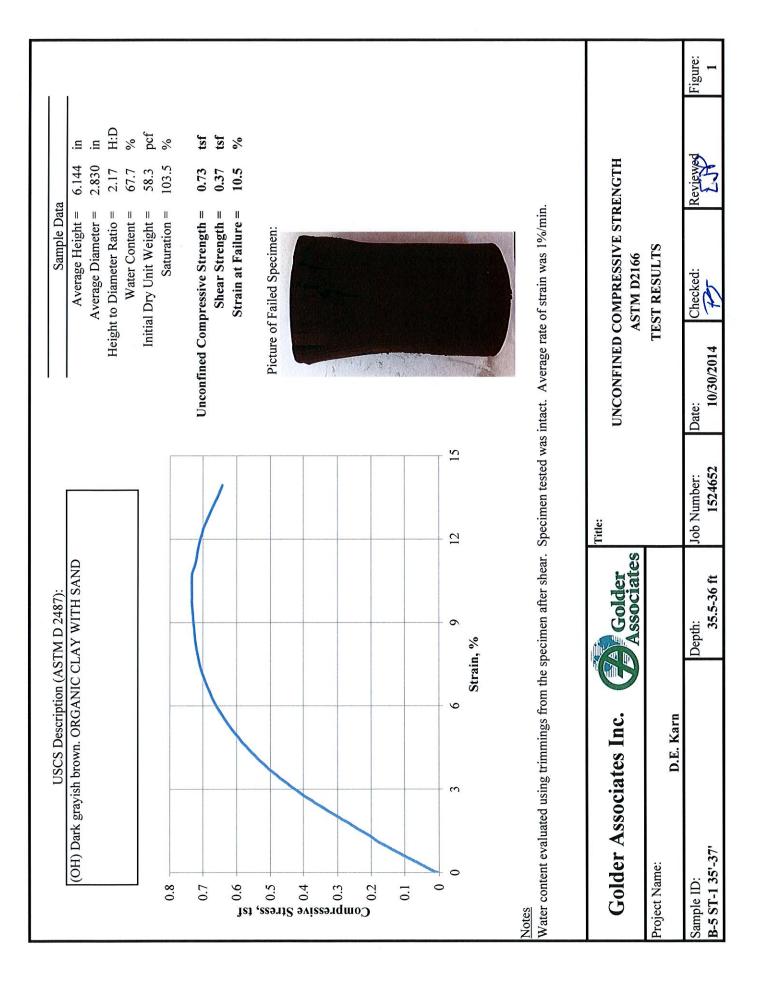
Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

TECH HD

DATE 3/13/2015

CHECK REVIEW





March-15 152-4652 PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS WET ASTM D421, D422, D4318 PROJECT NAME: D.E. Karn SAMPLE ID: B-4 ST-1 DEPTH (ft): 24.0-26.0 TYPE: Tube 3-inch 1.5-inch 1-inch 3/4-inch 3/8-inch #20 #60 #100 #200 100 90 80 70 Passing 60 50 Percent 40 30 20 10 0 100 10 1 0.1 0.01 0.001 Particle Size (mm) Particle Size (mm) Percentage Sieve % Passing Description 60 75.0 100.0 Cobbles 0.00 3-inch U-Line A-line 2-inch 50.8 100.0 1.5-inch 37.5 100.0 50 Coarse Gravel 0.00 Sieve Analysis Separation on No. 4 Sieve) 25.0 100.0 1-inch CH or OH 0.00 3/4-inch 19.0 100.0 Plasticity Index (PI) 30 1/2-inch 12.7 100.0 3/8-inch 9.5 100.0 Fine Gravel 0.00 4.75 100.0 #10 2.0 99.7 Coarse Sand 0.28 MH or OH (Initial #20 0.85 96.2 Medium Sand CL or OL #40 0.425 93.7 20.72 0.25 #60 90.9 10 #100 0.15 86.9 Fine Sand 14.40 #200 0.075 79.3 0.036 45.1 0 0.023 10 50 60 100 110 38.7 Liquid Limit (LL) 0.013 25.8 Silt or Clay 0.010 21.5 79.28 Fines 0.006 16.3 0.003 14.3 0.001 8.7 PL PI

USCS Description (ASTM D 2487):

Dark brown, ORGANIC CLAY WITH SAND

107 53 54 0.19

As-Received Moisture Content (%) 63.4

USCS Group Symbol OH

Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.



SPECIFIC GRAVITY OF SOIL SOLIDS

ASTM D 854

PROJECT NAME: D.E. Karn PROJECT NUMBER: 152-4652 SAMPLE ID: B-4 ST-1

SAMPLE TYPE: Tube	
SAMPLE PR	EPARATION
Wet (A) or Dry (B) B	% Passing #4 Sieve 100.0
•	
SAMPLE DESCRIPTION	NOTES
Dark brown, ORGANIC CLAY WITH SAND	
CALCUI	LATIONS
Test Temperature, T _t Density of Water @ T _t	18.5 °C 0.99850 g/mL
Pycnometer Number Mass of Pycnometer, Water, & Soil (M _{pws} @ T ₁)	3 371.76 g
Mass of Pycnometer, Water, & Soil (M _{pws} @ T _t) Mass of Pycnometer & Water (M _{pw} @ T _t)	350.95 g
Mass of Dish & Dry Soil	350.49 g
Mass of Dish Mass of Oven Dry Soil (M _s)	314.86 g 35.63 g
Specific Gravity (G _s @ T _t)	2.40
Temperature coefficient (K)	1.00030
Specific Gravity (G _s @ 20 °C)	2.40

	TECH	HD
Golder Associates Inc.	DATE	3/17/2015
Lansing, Michigan	CHECK	905
1=7	REVIEW	Edil



March-15 152-4652 PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS ASTM D421, D422, D4318 PROJECT NAME: D.E. Karn SAMPLE ID: B-9 DEPTH (ft): 8.5-10.0 TYPE: Jar 3-inch 1.5-inch 1-inch 3/4-inch 3/8-inch #10 #20 #60 #100 #200 100 90 80 70 60 Percent Passing 50 40 30 20 10 0 100 10 0.01 0.001 Particle Size (mm) Particle Size Percentage (mm) % Passing Description Sieve 3-inch Cobbles 0.00 60 75.0 100.0 U-Line 50.8 100.0 2-inch 1.5-inch 37.5 100.0 50 Coarse Gravel 0.00 Sieve Analysis (Initial Separation on No. 4 Sieve) 1-inch 25.0 100.0 CH or OH 11.36 3/4-inch 19.0 100.0 Plasticity Index (PI) 30 00 1/2-inch 12.7 100.0 3/8-inch 9.5 96.4 Fine Gravel 11.36 #4 4.75 88.6 #10 2.0 80.6 Coarse Sand 8.09 MH or OH #20 0.85 67.5 Medium Sand 22.51 #40 0.425 58.0 65.41 #60 0.25 49.6 10 34.81 #100 0.15 38.2 Fine Sand #200 0.075 23.2 CL -ML 0.038 15.3 0 100 110 10 20 40 50 60 70 80 90 0.025 10.9 Liquid Limit (LL) 0.014 7.7 Silt or Clay 23.23 0.010 6.5 Fines 0.007 4.6 0.004 2.9 0.002 2.1 Visual Description: Very dark gray, BOTTOM ASH **USCS Group Symbol** As-Received Moisture Content (%) 57.0

Notes: (1) Particle size analysis sample Mechanically dispersed using Stirring Apparatus A for about 1 Minute.

(2) Sample prepared for Atterberg Limits testing by the dry method. Material retained on No. 40 sieve removed from Atterberg Limits sample by dry sieving. Plastic Limit test performed by hand rolling. Method A Liquid Limit test performed using manual device.

TECH HD DATE 3/9/2015 CHECK REVIEW

ORGANIC CONTENT ASTM D2974, METHOD C

JOB NAME:

D.E. Karn

DATE:

March-15

JOB NUMBER:

152-4652

TECH: TDS

REVIEW: 530

	MOISTURE CONTENT DETERMINATION									
sample #	B-2	B-4	B-4	B-5	B-5	B-7				
depth (ft)	33.5-35.0	21.5-23.0	24.0-26.0	33.5-35.0	35.5-36.0	18.5-20.0				
tare #	1	2	2	3	1	4				
wt wet soil & tare (g)	181.14	169.55	165.85	172.19	178.12	162.52				
wt dry soil & tare (g)	142.41	131.44	141.00	124.70	175.37	116.05				
wt tare (g)	80.82	81.93	81.92	83.69	83.69	79.89				
wt lost (g)	38.73	38.11	24.85	47.49	2.75	46.47				
wt soil, dry (g)	61.59	49.51	59.08	41.01	91.68	36.16				
% moisture	62.9%	77.0%	42.1%	115.8%	3.0%	128.5%				
	ASH & OF	RGANIC CON	TENT DETER	RMINATION						
wt soil & tare, dry (g)	142.41	131.44	141.00	124.70	175.37	116.05				
wt soil & tare, burnt (g)	137.16	125.71	132.77	116.90	168.00	108.93				
wt tare (g)	80.82	81.93	81.92	83.69	83.69	79.89				
wt lost (g)	5.25	5.73	8.23	7.80	7.37	7.12				
wt soil, dry (g)	61.59	49.51	59.08	41.01	91.68	36.16				
% ash	91.5%	88.4%	86.1%	81.0%	92.0%	80.3%				
% Volatile organics	8.5%	11.6%	13.9%	19.0%	8.0%	19.7%				

Note: Gravel removed from test specimen prior to moisture content determination

Furnace temperature for ash content determination was approximately 450° C

GOLDER ASSOCIATES LANSING, MI

MOISTURE CONTENT DETERMINATIONS

Project Number Project Name

152-4652 D.E. Karn Bottom Ash Tank

Tech
Date
3/15/2015
Checked
Reviewed

					·	Reviewed	61	28
Borehole	Comple	Sample	Wt. of Wet				Weight of	Water
Number	Sample	Number		Soil & Tare	0	Weight of	Dry Soil (g)	Content
	Depth (ft)		(g)	(g)	Tare (g)	Water (g)	Dry Soli (g)	(%)
B-1 B-1	3.5-5.0	-	62.92	50.44	13.51	12.48	36.93	33.8
B-1	14.5-14.9	-	54.49	39.65	13.71	14.84	25.94	57.2
B-1	24.5-25.0	-	69.40	61.57	13.82	7.83	47.75	16.4
	28.5-30.0	-	66.12	55.88	13.64	10.24	42.24	24.2
B-1	33.5-35.0		72.20	62.65	13.52	9.55	49.13	19.4
B-2 B-2	3.5-5.0	-	33.11	28.30	14.76	4.81	13.54	35.5
B-2 B-2	8.5-10.0	-	35.31	30.93	13.46	4.38	17.47	25.1
B-2	13.5-14.5	A	50.47	38.78	14.82	11.69	23.96	48.8
B-2	14.5-15.0	-	43.71	25.70	13.56	18.01	12.14	148.4
B-2	21.0-22.5	_	67.67	57.29	14.85	10.38	42.44	24.5
	23.5-24.0		78.54	61.05	13.59	17.49	47.46	36.9
B-2	28.5-30.0	-	95.85	82.67	13.72	13.18	68.95	19.1
B-2	33.5-35.0	-	43.66	31.80	13.76	11.86	18.04	65.7
B-2	35.0-35.5		82.36	71.98	13.75	10.38	58.23	17.8
B-3	1.0-2.5	-	45.99	39.80	13.73	6.19	26.07	23.7
B-3	3.5-5.0		75.31	68.24	13.79	7.07	54.45	13.0
B-3	8.5-10.0	-	105.35	87.44	13.67	17.91	73.77	24.3
B-3	13.5-15.0	-	53.81	44.51	13.74	9.30	30.77	30.2
B-3	18.5-20.0	-	63.33	53.68	14.97	9.65	38.71	24.9
B-3	23.5-25.0	-	51.22	43.28	13.53	7.94	29.75	26.7
B-4	1.0-2.5	-	74.39	66.43	13.61	7.96	52.82	15.1
B-4	3.5-5.0	-	63.68	57.87	13.83	5.81	44.04	13.2
B-4	8.5-10.0	-	72.15	64.87	13.86	7.28	51.01	14.3
B-4	13.5-15.0		58.99	51.15	14.73	7.84	36.42	21.5
B-4	18.5-20.0	-	57.56	49.84	13.86	7.72	35.98	21.5
B-4	20.0-21.5	-	62.73	51.94	13.59	10.79	38.35	28.1
B-4	33.5-35.0	-	54.38	47.71	13.72	6.67	33.99	19.6
B-5	1.0-2.5	-	39.38	35.87	13.60	3.51	22.27	15.8
B-5	3.5-5.0	, ' :-)	42.57	37.40	13.69	5.17	23.71	21.8
B-5	6.0-7.5	-	36.45	30.87	13.50	5.58	17.37	32.1
B-5	9.5-10.0	-	43.22	31.18	13.71	12.04	17.47	68.9
B-5	12.0-15.5	-	36.45	30.87	13.50	5.58	17.37	32.1
B-5	23.5-25.0	-	64.61	49.50	13.86	15.11	35.64	42.4
B-5	28.3-29.5	-	48.11	38.45	13.86	9.66	24.59	39.3
B-5	29.5-30.0	-	45.83	39.48	14.80	6.35	24.68	25.7
B-5	33.5-35.0	-	36.36	25.99	13.60	10.37	12.39	83.7
B-5	37.5-39.0	-	43.49	39.14	13.71	4.35	25.43	17.1
B-6	3.5-5.0	-	42.52	38.81	13.74	3.71	25.07	14.8
B-6	8.5-10.0	-	40.60	37.17	13.56	3.43	23.61	14.5
B-6	13.5-15.0	-	46.72	42.61	13.62	4.11	28.99	14.2
B-6	18.5-20.0	-	46.96	40.02	13.60	6.94	26.42	26.3
		Gold	er Associa	ates - Lans	sing Mich	igan		

MOISTURE CONTENT DETERMINATIONS

Project Number 152-4652 Tech HD **Project Name** D.E. Karn Bottom Ash Tank 3/15/2015 Date Checked 205 Reviewed CARS Wt. of Wet | Wt. of Dry Water Sample Weight of Borehole Sample Soil & Tare | Soil & Tare Weight of Weight of Content Number Dry Soil (g) Number Depth (ft) (g) (g) Tare (g) Water (g) (%) B-7 3.5-5.0 62.92 53.21 13.71 9.71 39.50 24.6 **B-7** 8.5-10.0 45.85 38.17 -13.00 7.68 25.17 30.5 **B-7** 13.5-15.0 45.89 -39.85 13.75 6.04 26.10 23.1 B-7 18.5-20.0 32.32 23.24 13.81 9.08 9.43 96.3 B-7 20.0-21.5 47.84 -42.13 14.86 5.71 27.27 20.9 B-8 3.5-5.0 -31.94 25.16 13.51 6.78 11.65 58.2 B-8 8.5-10.0 52.66 45.18 13.61 7.48 31.57 23.7 B-8 13.5-15.0 49.41 -41.59 13.64 7.82 27.95 28.0 **B-8** 18.5-20.0 44.37 39.63 14.91 4.74 24.72 19.2 B-8 23.5-25.0 65.96 61.02 13.85 4.94 47.17 10.5 B-8 25.0-26.5 -55.37 50.99 13.61 4.38 37.38 11.7 B-9 3.5-5.0 40.39 30.78 14.80 9.61 15.98 60.1 B-9 13.5-14.5 52.65 40.82 13.92 11.83 26.90 44.0 B-9 14.5-15.0 -36.92 30.32 14.85 6.60 15.47 42.7 B-9 18.5-20.0 54.87 47.71 13.91 7.16 33.80 21.2 B-9 23.5-25.0 67.56 62.54 13.56 5.02 48.98 10.2 B-9 28.5-30.0 -45.33 41.61 13.73 3.72 27.88 13.3 Golder Associates - Lansing Michigan